

Use of Recycled Crushed Concrete (RCC) Fines for Potential Soil Stabilization

Nebraska Department of Roads

Research Project: Use of Recycled Crushed Concrete (RCC) Fines for Potential Soil Stabilization

Location: Material & Research

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Hwy 75 Waste RCC



Hwy 75 Waste RCC

OBJECTIVE OF THE INVESTIGATION:

This study evaluated the use of Recycled Crushed Concrete (RCC) Fines for potential soil stabilization. Soil stabilization is the enhancement of subgrade stability to improve the constructability of successive pavement layers. Use of RCC fines may not only provide less costly alternatives for subgrade stabilization, but their use may also alleviate landfill disposal challenges.

DESCRIPTION OF INVESTIGATION:

- 1. Evaluate the performance/effectiveness of laboratory mix design procedures in predicting field performance.
- 2. Evaluate Soil Liquid Limit (LL) per AASHTO T 89, Plastic Limit (PL) and Plasticity Index (PI) in accordance with AASHTO T 90, of virgin and lime stabilized soils.
- 3. Evaluate the unconfined compressive strength (q_u) in accordance with AASHTO T 208 of virgin and lime stabilized soils.

LABORATORY INVESTIGATION

Tasks proposed for this investigation:

Mechanical Evaluation

• Components of Stabilization by Task

<u>Task 1:</u>

Soil Gradation

- Virgin Soils
- Soils with RCC
- Soils with lime and RCC

<u>Task 2:</u>

PH Level

- Soils of high, medium and low plasticity with Lime in accordance with ASTM C 977 "Standard Specification for Quicklime and Hydrated Lime for Soil
 - 977 "Standard Specification for Quicklime and Hydrated Lime for S Specification".
- Soils of high, medium and low plasticity with Lime and RCC material in accordance with ASTM C 977 "Standard Specification for Quicklime and Hydrated Lime for Soil Specification".

<u>Task 3</u>:

Mechanical Properties

Evaluation of mechanical properties of High, Medium and Low Plasticity Soils with Lime and RCC

- Decreased cohesiveness (Plasticity)
- Decreased volume expansion or compressibility
- Increased strength

LABORATORY TESTING:

As described in the laboratory investigations under Task 1, the investigation proceeded first with the gradation evaluation of virgin soils of High, Medium and Low PI. Table 1 provides the sieve analysis of the Waste RCC and virgin soils used in the investigation.

| Sieve Analysis Total Percent Passing | | | | | | | | | |
|---|-----------------------|--------|------|-------|-------|-------|-------|-------|--------|
| Soil Index | Sample ID- | No.3/8 | No.4 | No.10 | No.30 | No.40 | No.50 | N.100 | No.200 |
| NA | Waste RCC - Hwy 31 | 100 | 98 | 89 | 56 | 45 | 36 | 20 | 9 |
| NA | Waste RCC -Hwy 75 | 99 | 88 | 56 | 32 | 28 | 25 | 17 | 13 |
| Low | Virgin Soil-Low PI | - | - | 100 | 99 | 98 | 97 | 94 | 92 |
| Medium | Virgin Soil-Medium Pl | - | - | - | 100 | 99 | 99 | 99 | 98 |
| High | Virgin Soil- High Pl | - | - | 100 | 100 | 100 | 99 | 98 | 97 |

Table 1.

Table 2. shows the sieve analysis performed on soils with Waste RCC added and with Waste RCC and lime added. Sieve analysis results show that the addition of the 3% Waste RCC does not change the gradation of the virgin materials significantly. However, with the addition of lime the percent passing the #30 through #200 sieve sizes shows a change. Different percentages of lime were used depending on results of pH level evaluation shown in Table 3. The percent lime added to the medium and high PI soil was 4% and for the low soil index was 2%. It is clear that the lime changed the gradation, causing the material to become coarser.

Table 2.

| Sieve Analysis Materials Combinations- Total Percent Passing | | | | | | | | | |
|--|-------------------------------------|--------|------|-------|-------|-------|-------|-------|--------|
| Soil Index | Sample ID- | No.3/8 | No.4 | No.10 | No.30 | No.40 | No.50 | N.100 | No.200 |
| Low | Soil w/ 3% RCC- Hwy 31 | - | - | 100 | 99 | 97 | 95 | 91 | 89 |
| | Soil w/ 3% RCC- Hwy 75 | - | - | 100 | 98 | 97 | 95 | 91 | 89 |
| | Soil w/ 3% RCC - Hwy 31 and 2% lime | - | - | 100 | 97 | 95 | 90 | 81 | 75 |
| | Soil w/ 3% RCC - Hwy 75 and 2% lime | - | - | 100 | 97 | 93 | 88 | 79 | 73 |
| | | | | | | | | 1 | |
| Medium | Soil w/ 3% RCC- Hwy 31 | - | - | 100 | 99 | 99 | 98 | 97 | 97 |
| | Soil w/ 3% RCC- Hwy 75 | - | - | 100 | 98 | 98 | 98 | 97 | 97 |
| | Soil w/ 3% RCC - Hwy 31 and 4% lime | - | - | 100 | 81 | 76 | 69 | 62 | 58 |
| | Soil w/ 3% RCC — Hwy 75 and 4% lime | - | - | 100 | 79 | 72 | 67 | 60 | 57 |
| | | | | | | | | | |
| High | Soil w/ 3% RCC- Hwy 31 | - | - | 100 | 99 | 99 | 98 | 97 | 95 |
| | Soil w/ 3% RCC- Hwy 75 | - | - | 100 | 98 | 98 | 97 | 96 | 95 |
| | Soil w/ 3% RCC - Hwy 31 and 4% lime | - | - | 100 | 68 | 60 | 52 | 41 | 33 |
| | Soil w/ 3% RCC - Hwy 75 and 4% lime | - | - | 100 | 83 | 75 | 66 | 51 | 43 |

Part of the study consisted of evaluating the virgin soil chemical PH level for each soil type, with the addition of the 3% waste RCC material, and with lime and with waste RCC material. The soil pH was tested in accordance with ASTM C 977. Different percentages of lime were added to the soil in order to reach a pH soil level of 12.40 to stabilize the soil. The testing for pH levels is used to determine the percent lime that is required for the soil type; in order, to increase the unconfined compressive strength and decrease plastic index. The following Table 3 shows the results of the materials tested.



Figure 1. PH Level Soil Samples



Figure 2. PH Level Testing

| Lime (%) | LOW PI Soil | | | | | | | |
|-------------------|-----------------------|---------------------------|--------------------------|--|--|--|--|--|
| Added | LOW PI Virgin Soil | Soil with 3% RCC - Hwy75 | | | | | | |
| Autou | | PH (ASTM C 977) | | | | | | |
| 0 | 7.48 | 9.33 | 8.18 | | | | | |
| 2 | 12.55 | 12.51 | 12.50 | | | | | |
| 3 | 12.60 | 12.60 | 12.56 | | | | | |
| 4 | 12.64 | 12.65 | 12.59 | | | | | |
| 5 | 12.66 | 12.66 | 12.61 | | | | | |
| 6 | 12.66 | 12.68 | 12.65 | | | | | |
| | | MEDIUM PI Soil | | | | | | |
| Lime (%) | Medium PI Virgin Soil | Soil with 3% RCC - Hwy 31 | Soil with 3% RCC -Hwy 75 | | | | | |
| Added | | PH (ASTM C 977) | | | | | | |
| 0 | 8.49 | 9.77 | 8.99 | | | | | |
| 2 | 12.41 | 12.44 | 12.41 | | | | | |
| 3 | 12.45 | 12.50 | 12.46 | | | | | |
| 4 | 12.51 | 12.50 | 12.49 | | | | | |
| 5 | 12.51 | 12.52 | 12.51 | | | | | |
| 6 | 12.52 | 12.53 | 12.52 | | | | | |
| 1. (0/) | | HIGH PI Soil | | | | | | |
| Lime (%) Added | HIGH PI Virgin Soil | Soil with 3% RCC - Hwy 31 | Soil with 3% RCC -Hwy 75 | | | | | |
| Auueu | | PH (ASTM C 977) | | | | | | |
| 0 | 8.01 | 8.26 | 8.50 | | | | | |
| 2 | 12.32 | 12.20 | 12.22 | | | | | |
| 3 | 12.44 | 12.39 | 12.39 | | | | | |
| 4 | 12.47 | 12.44 | 12.43 | | | | | |
| 5 | 12.49 | 12.47 | 12.46 | | | | | |
| 6 | 12.50 | 12.49 | 12.49 | | | | | |
| | | | | | | | | |

Table 3.

The University of Texas at Arlington defined the consistency of an unconfined soil (q_u) as very soft to very stiff as shown in figure 3. The soils tested in this study were found to be very soft as shown in Table 4.

| Very stiff | 4000-8000 |
|-------------|---------------------------|
| Stiff | 2000-4000 |
| Medium | 1000-2000 |
| Soft | 500-1000 |
| Very soft | 0-500 |
| Consistency | q_u (lb/ft ² |

Figure 3.



Figure 4. Failure pattern typical of brittle specimens.

Atterberg Limits (LL, PL, PI), percent retained on the #200 sieve, Maximum Dry Density and Optimum Moisture Content, and Unconfined Compression (q_u) tests were performed in order to evaluate the mechanical properties. These tests were performed on all soil mixture combinations proposed in this research and are provided in Table 4. It is observed that the addition of the 3% waste RCC had a minimal effect the LL, PL, and PI, Maximum Dry Density and Optimum Moisture content for all the soil types except for the High PI soils where the Optimum Moisture decreased 1 to 2 percentage points. When lime was added the LL, PL, and PI, and Maximum Dry Density and Optimum Moisture content results changed as would be expected. When evaluating the unconfined compressive strength (q_u) results with the addition of RCC and compared to the original soil, the key finding were found as follows.

KEY FINDINGS

The results show that the compressive strength of the soil with 3% RCC waste material did not cause a detrimental effect to soil modification. Figure 5. shows a typical stockpile of RCC fines produced from crushing operations. The Department of Roads has limited the amount of fines to be used as base course, due to potential drainage problems. When the concrete to be crushed is severely deteriorated the amount of concrete fines could be high; this may cause the Contractor to haul concrete fines to a landfill for disposal.



Figure 5. Recycled Crushed - Concrete Fines

Table 4.

| Sample ID- Project | No.200 Passing Material | Liquid Limit (LL) (%) | Plastic Limit (PL) (%) | Plasticity Index (PI) (%) | Moisture Density Ib\ft ³ | Optimum Moisture (%) | Unconfined (q _u) psi | | | | | |
|---|-------------------------------|----------------------------------|---------------------------------|---------------------------------|---|----------------------------|-------------------------------------|--|--|--|--|--|
| | | AASHTO T89 AASHTO T90 AASHTO T99 | | | | | AASHTO T208 | | | | | |
| | | | | Total averaged of fo | our samples per testing |] | | | | | | |
| | | | LOW PI SOIL | | | | | | | | | |
| Virgin Soil | 92 | 30 | 16 | 14 | 110.2 | 15.3 | 184 | | | | | |
| Soil with 3% RCC - Hwy 31 | 89 | 30 | 16 | 14 | 111.3 | 15.2 | 215 | | | | | |
| Soil with 3% RCC -Hwy 75 | 89 | 30 | 16 | 14 | 111.3 | 15.4 | 212 | | | | | |
| Soil w/ 3% RCC - Hwy 31 and 2% lime | 75 | 30 | 25 | 5 | 102.3 | 17.4 | 229 | | | | | |
| Soil w/ 3% RCC - Hwy 75 and 2% lime | 73 | 32 | 25 | 7 | 102.3 | 17.7 | 227 | | | | | |
| | | | | MEDIUM PI SOI | L | | | | | | | |
| Virgin Soil | 98 | 46 | 18 | 28 | 104.5 | 19.2 | 148 | | | | | |
| Soil with 3% RCC - Hwy 31 | 97 | 45 | 18 | 27 | 104.7 | 19.0 | 166 | | | | | |
| Soil with 3% RCC -Hwy 75 | 97 | 46 | 17 | 29 | 104.8 | 19.0 | 148 | | | | | |
| Soil w/ 3% RCC - Hwy 31 and 4% lime | 58 | 37 | 30 | 7 | 95.3 | 21.3 | 154 | | | | | |
| Soil w/ 3% RCC - Hwy 75 and 4% lime | 57 | 37 | 31 | 6 | 95.1 | 21.3 | 149 | | | | | |
| | HIGH PI SOIL | | | | | | | | | | | |
| Virgin Soil- | 97 | 70 | 28 | 42 | 94.2 | 25.3 | 148 | | | | | |
| Soil with 3% RCC - Hwy 31 | 95 | 70 | 28 | 42 | 93.8 | 22.9 | 148 | | | | | |
| Soil with 3% RCC -Hwy 75 | 95 | 70 | 28 | 42 | 94.8 | 23.9 | 134 | | | | | |
| Soil w/ 3% RCC - Hwy 31 and 4% lime | 33 | 44 | 39 | 5 | 91.5 | 27.0 | 159 | | | | | |
| Soil w/ 3% RCC - Hwy 75 and 4% lime | 43 | 44 | 39 | 5 | 90.9 | 26.7 | 151 | | | | | |

SUMMARY:

This study examined the use of Waste RCC for potential soil modification. The major findings in this evaluation were as follows:

- The addition of 3% RCC Fines did not change the amount of lime required for modification of the three soils tested.
- The 3% RCC Fines was found to be inert as either a stabilizer or a short-term modifier for all three soil types.

FOLLOW UP FIELD IMPLEMENTATION:

A limited field investigation should be performed to assess the in-situ performance of stabilized subgrades with the addition of RCC Fines. Based on the results to date, NDOR-Material and Research Division will allow an option to use up to 3% RCC fines in the subgrade. A comparison summary with estimated savings due to the addition of RCC fines will be evaluated and reported.